

ΩE

SHEET NO.

OF

OUTFALL-1-TRANSIT CENTER-REV1 Type II 24-hr 1 year Rainfall=2.53" Printed 11/17/2020 Prepared by DEWBERRY HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC Page 1

Summary for Pond Bio-1: BIO-1

2.110 ac, 82.46% Impervious, Inflow Depth = 1.99" for 1 year event 6.76 cfs @ 11.98 hrs. Volume= 0.350 af 0.064 af, Atten= 98%, Lag= 239.2 min 0.12 cfs @ 15.96 hrs, Volume= Outflow = 0.12 cfs @ 15.96 hrs, Volume=

Routing by Dyn-Stor-Ind method, Time Span= 0.00-54.00 hrs, dt= 0.03 hrs Peak Elev= 290.35' @ 15.96 hrs Surf.Area= 6,945 sf Storage= 12.677 cf

Plug-Flow detention time= 558.8 min calculated for 0.064 af (18% of inflow)

Center-of-Mass det. time= 372.4 min (1,158.3 - 785.9)

Primary =

Invert Avail.Storage Storage Description Volume 282.50' 17,697 cf Custom Stage Data (Prismatic)Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
282.50	2,073	0.0	0	0
283.50	2,489	40.0	912	912
285.00	3,121	40.0	1,683	2,595
285.67	3,409	40.0	875	3,470
286.00	3,552	25.0	287	3,758
288.00	4,434	25.0	1,997	5,754
289.50	5,115	25.0	1,790	7,544
290.00	6,258	100.0	2,843	10,388
290.50	7,248	100.0	3,377	13,764
291.00	8,482	100.0	3,933	17,697

Invert Outlet Devices Device Routing

282.50' 18.0" Round Culvert #1 Primary

L= 35.4' RCP, rounded edge headwall, Ke= 0.100

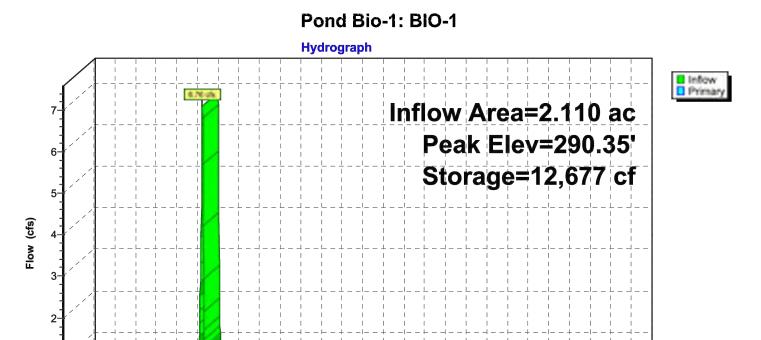
Inlet / Outlet Invert= 282.50' / 282.25' S= 0.0071 '/' Cc= 0.900 n= 0.013. Flow Area= 1.77 sf

290.32' **26.0"** x **26.0"** Horiz. Orifice/Grate C= 0.600 #2 Device 1 Limited to weir flow at low heads

OUTFALL-1-TRANSIT CENTER-REV1

Type II 24-hr 1 year Rainfall=2.53"

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OUTFALL-1-TRANSIT CENTER-REV1 Type II 24-hr 1 year Rainfall=2.53' Printed 11/17/2020 Prepared by DEWBERRY

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54

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Storage Elevation Primary

Hydrograph for Pond Bio-1: BIO-1

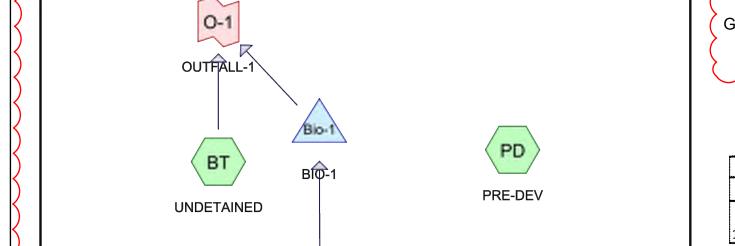
iiiiie	ILITION	Glorage	Lievation	i iiiiiai y
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs
0.00	0.00	0	282.50	0.00
1.50	0.00	0	282.50	0.00
3.00	0.00	0	282.50	0.00
4.50	0.01	13	282.52	0.00
6.00	0.03	106	282.63	0.00
7.50	0.05	304	282.85	0.00
9.00	0.09	652	283.23	0.00
10.50	0.18	1,311	283.89	0.00
12.00	6.47	7,208	289.23	0.00
13.50	0.26	11,421	290.16	0.00
15.00	0.16	12,465	290.32	0.0
16.50	0.11	12,670	290.35	0.12
18.00	0.09	12,650	290.34	0.10
19.50	0.08	12,629	290.34	0.08
21.00	0.07	12,614	290.34	0.07
22.50	0.06	12,609	290.34	0.06
24.00	0.06	12,604	290.34	0.06
25.50	0.00	12,513	290.32	0.00
27.00	0.00	12,500	290.32	0.00
28.50	0.00	12,496	290.32	0.00
30.00	0.00	12,494	290.32	0.00
31.50	0.00	12,493	290.32	0.00
33.00	0.00	12,493	290.32	0.00
34.50	0.00	12,493	290.32	0.00
36.00	0.00	12,492	290.32	0.00
37.50	0.00	12,492	290.32	0.00
39.00	0.00	12,492	290.32	0.00
40.50	0.00	12,492	290.32	0.00
42.00	0.00	12,492	290.32	0.00
43.50	0.00	12,492	290.32	0.00
45.00	0.00	12,492	290.32	0.00
46.50	0.00	12,492	290.32	0.00
48.00	0.00	12,492	290.32	0.00
49.50	0.00	12,492	290.32	0.00
51.00	0.00	12,492	290.32	0.00
52.50	0.00	12,492	290.32	0.00

12,492

290.32

54.00

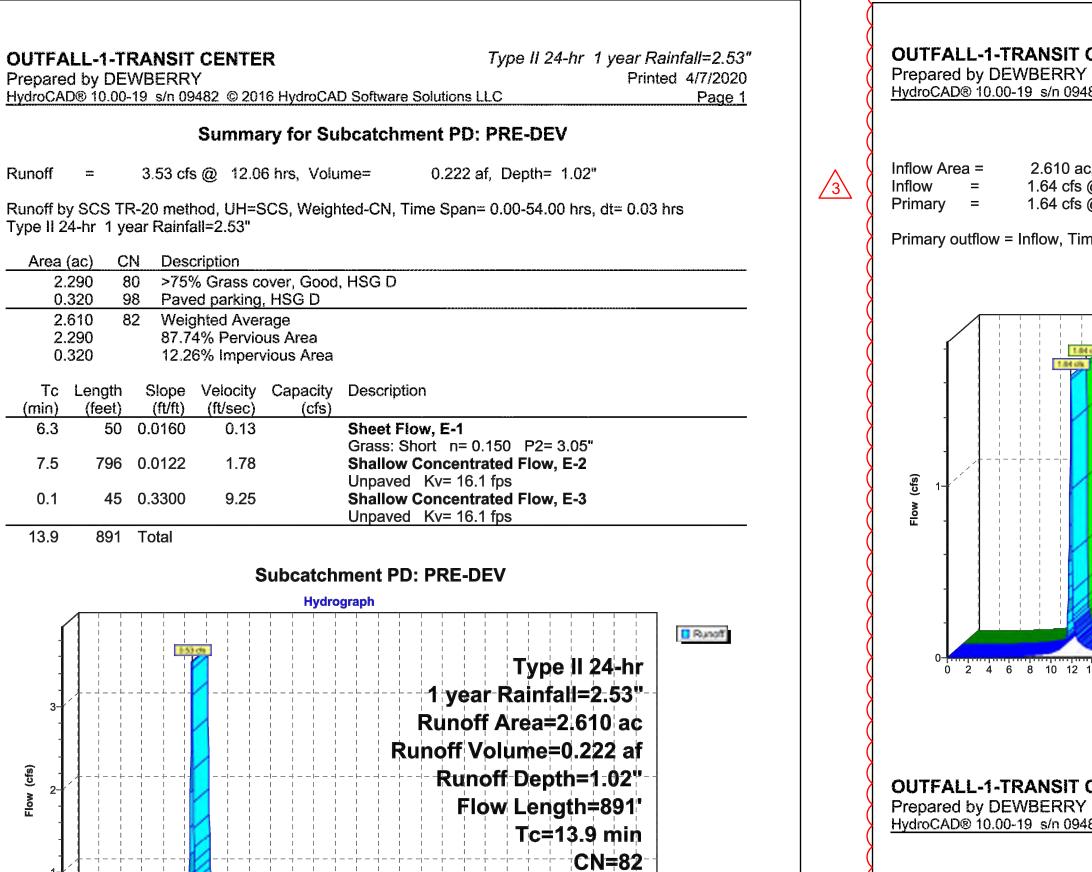
0.00



Routing Diagram for OUTFALL-1-TRANSIT CENTER Prepared by DEWBERRY. Printed 4/8/2020

OUTFALL

1-YEAR STORM EVENT-POST DEVELOPMENT ROUTING



1-YEAR STORM EVENT-PRE DEVELOPMENT ROUTING

1-YR ENERGY BALANCE-OUTFALL NARRATIVE

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54

PROJECT DESCRIPTION

Page 3

THE IMPROVEMENTS PROPOSED WITH THIS PLAN ARE PART OF A PREVIOUSLY APPROVED PLAN CALLED 'WASHINGTON DULLES INTERNATIONAL AIRPORT, REMOTE EMPLOYEE PARKING PHASE 2". THE IMPROVEMENTS PROPOSED WITH THIS PLAN ARE THEREFORE CONSIDERED A RE-DEVELOPMENT. THE STUDY AREA FOR OUTFALL 1 TOTALS 2.61 ACRES (2.11 AC TO BIORETENTION) BASIN, AND 0.50 AC OF UNDETAINED BYPASS). THE AREA OF PROPOSED IMPROVENTS, 2.61 ACRES, HAS BEEN MANAGED FOR WATER QUALITY AND QUANTITY IN ACCORDANCE WITH TECHNICAL CRITERIA SET FORTH IN THE VIRGINIA STORMWATER MANAGEMENT PROGRAM PERMIT REGULATIONS (9VAC25-870-10 et seq.). NON-PROPRIETARY BMP'S HAVE BEEN DESIGNED TO MEET SPECIFICATIONS OUTLINED IN THE VIRGINIA DEQ 2013 STORMWATER BMP CLEARNINGHOUSE STANDARDS AND SPECIFICATIONS. REFER TO BMP NARATIVE ON SHEET C.07.203 FOR THIS PROJECT. THE CALCULATIONS SHOWN ON THIS SHEET AND REFERENCED ON THIS NARRATIVE RELATE ONLY TO THE 1-YR ENERGY BALANCE REQUIREMENT, FOR ADEQUATE OUTFALL RELATING TO THE THE 2-YR AND 10-YR STORM EVENTS SEE SHEETS C.06.204 & C.06.205.

1YR-ENERGY BALANCE NARRATIVE

THE BIORETENTION PROPOSED ON THIS PROJECT HAS BEEN DESIGNED TO CAPTURE RUNOFF FROM 2.11 RE-DEVELOPED ACRES AND DISCHARGE INTO EXISTING CONCRETE OUTFALL CHANNEL THE RUNOFF IS ATTENUATED SUCH THAT THE TOTAL 1 YEAR RELEASE RATE FROM THE BIORETENTION BASIN AND UNDETAINED AREA MEET THE DISCHARGE REQUIRED BY ENERGY BALANCE METHODOLOGY FOR THE 2.61 ACRE STUDY AREA. SINCE THE DRAINAGE AREA IS OVER 1 ACRE AN IMPROVEMENT FACTOR OF 0.8 WAS USED IN THE ENERGY BALANCE CALCULATIONS. (SEE THIS SHEET FOR ENERGY BALANCE CALCULATIONS)

THE CALCULATED PRE-DEVELOPMENT 1-YEAR FLOW GENERATED FROM THE STUDY AREA TO THE EXISTING CONCRETE CHANNEL IS 3.53 CFS, SEE SHEET C.06.100 FOR PRE-DEVELOPMENT AREAS, CN & TC CALCULATIONS. SEE SHEET C-06.202 FOR PRE-DEVELOPMENT ROUTING CALCULATIONS.

THE POST-DEVELOPMENT, COMBINED UNDETAINED AND DETAINED AREAS TOTAL 2.61 AC; SEE SHEET C-06.200 FOR PROPOSED AREAS, ADJUSTED CN & TC CALCULATIONS. THE FINAL CALCULATED 1-YEAR FLOW DISCHARGED FROM THE PROPOSED DEVELOPMENT IS 1.64 CFS, SEE SHEET C.06.205 FOR ROUTING CALCULATIONS. SINCE THE DEVELOPED DISCHARGE OF 1.64 CFS. IS EQUAL OR LESS THAN THE CALCULATED ALLOWABLE RATE OF 1.81 CFS, THE REQUIREMENT OF THE ENERGY BALANCE METHODOLOGY IS MET, SEE SHEET C.06.205.

SINCE OUTFALL 1 DISCHARGES MEET THE 1-YEAR ENERGY BALANCE (SHEET C.06.205), WE BELIEVE THAT ADEQUATE STORMWATER MANAGEMENT AND ADEQUATE OUTFALL FOR THIS DRAINAGE AREA HAS BEEN PROVIDED WITH THE PROPOSED IMPROVEMENTS.

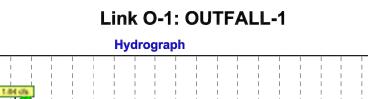
> NOTE: SOILS ARE NOT DEFINED IN THE LOUDOUN COUNTY INTERPRETIVE GUIDE TO THE USE OF SOIL MAPS. SOIL TYPE 'D' IS ASSUMED FOR ALL CALCULATIONS. SEE GEOTECHNICAL REPORT SUBMITTED BY TERRACON: Geotechnical Engineering Report, Dulles North Park and Ride, Sterling, Virginia. June 23, 2020. Terracon Project No. JD205034

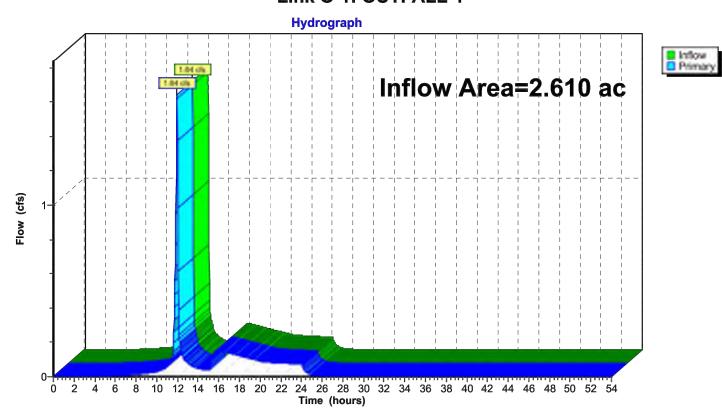
Type II 24-hr 1 year Rainfall=2.53" **OUTFALL-1-TRANSIT CENTER-REV1** Printed 11/16/2020 HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC Page 1

Summary for Link O-1: OUTFALL-1

2.610 ac, 81.23% Impervious, Inflow Depth = 0.66" for 1 year event 1.64 cfs @ 11.96 hrs, Volume= 1.64 cfs @ 11.96 hrs, Volume= 0.143 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-54.00 hrs, dt= 0.03 hrs





OUTFALL-1-TRANSIT CENTER-REV1 Type II 24-hr 1 year Rainfall=2.53" Printed 11/16/2020 Prepared by DEWBERRY HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC

Hydrograph for Link O-1: OUTFALL-1

(hours)

Primary Time Inflow Elevation

Inflow Elevation

(cfs) (feet)

(110010)	(010)	(1001)		(Hours)	10107	(1000)	10107
0.00	0.00	0.00	0.00	31.20	0.00	0.00	0.00
0.60	0.00	0.00 0.00 0.00	0.00	31.80	0.00 0.00 0.00	0.00	0.00 0.00 0.00
1.20	0.00	0.00	0.00	32.40	0.00	0.00	0.00
1.80	0.00	0.00	0.00	33.00	0.00	0.00	0.00
2.40	0.00	0.00	0.00	33.60	0.00	0.00	0.00
3.00	0.00	0.00 0.00 0.00	0.00 0.00 0.00	34.20	0.00 0.00 0.00	0.00	0.00 0.00 0.00
3.60	0.00	0.00	0.00	34.80	0.00	0.00	0.00
4.20	0.00	0.00	0.00	35.40	0.00	0.00	0.00
4.80	0.00	0.00	0.00	36.00	0.00	0.00	0.00
5.40	0.00	0.00	0.00	36.60	0.00	0.00	0.00
6.00	0.00	0.00 0.00 0.00	0.00 0.00 0.01	37.20	0.00 0.00 0.00 0.00	0.00	0.00 0.00 0.00
6.60	0.01	0.00	0.01	37.80	0.00	0.00	0.00
7.20	0.01	0.00	0.01	38.40	0.00	0.00	0.00
7.80	0.01	0.00	0.01	39.00	0.00	0.00	0.00
8.40	0.01	0.00 0.00 0.00	0.01 0.01 0.02	39.00 39.60 40.20	0.00 0.00	0.00	0.00 0.00 0.00
9.00	0.02	0.00	0.02	40.20	0.00	0.00	0.00
9.60	0.02	0.00	0.02	40.80	0.00	0.00	0.00
10.20	0.03	0.00	0.03	41.40	0.00	0.00	0.00
10.80	0.05	0.00	0.02 0.03 0.05	42.00	0.00 0.00 0.00	0.00	0.00
11.40	0.09	0.00 0.00 0.00	0.09	42.60	0.00	0.00	0.00 0.00 0.00
12.00	1.38	0.00	1.38	43.20	0.00	0.00	0.00
12.60	0.10	0.00 0.00 0.00 0.00	1.38 0.10 0.07	43.20 43.80 44.40 45.00	0.00	0.00	0.00 0.00 0.00
13.20	0.07	0.00	0.07	44.40	0.00 0.00	0.00	0.00
13.80	0.05	0.00	0.05	45.00	0.00	0.00	0.00
14.40	0.04	0.00	0.04	45.60	0.00	0.00	0.00
15.00	0.04	0.00	0.04	46.20	0.00	0.00	0.00
15.60	0.14	0.00 0.00 0.00	0.04 0.04 0.14	45.60 46.20 46.80	0.00 0.00	0.00	0.00 0.00 0.00
16.20	0.15	0.00	0.15 I	47.40	0.00	0.00	0.00
16.80	0.14	0.00 0.00 0.00	0.14 0.13 0.12	48.00 48.60 49.20	0.00 0.00 0.00	0.00	0.00 0.00 0.00
17.40	0.13	0.00	0.13	48.60	0.00	0.00	0.00
18.00	0.12	0.00	0.12	49.20	0.00	0.00	0.00
18.60	0.11	0.00	0 11 l	49.80	0.00	0.00	0.00
19.20	0.10	0.00	0.10	50.40	0.00	0.00	0.00
19.80	0.09	0.00 0.00 0.00	0.10 0.09 0.08	50.40 51.00 51.60	0.00 0.00 0.00	0.00	0.00 0.00 0.00
20.40	0.08	0.00 0.00	0.08	51.60	0.00	0.00	0.00
21.00	0.08	0.00	0.08	52.20	0.00	0.00	0.00
21.60	80.0	0.00	0.08	52.80	0.00	0.00	0.00
22.20	0.08	0.00 0.00 0.00	80.0 80.0 80.0	52.80 53.40 54.00	0.00 0.00 0.00	0.00 0.00 0.00	0.00
22.80	0.08	0.00	0.08	54.00	0.00	0.00	0.00
23.40	0.07	0.00 0.00 0.00 0.00	0.07				
24.00	0.07	0.00	0.07				
23.40 24.00 24.60	0.02	0.00	0.02				
25 20	0.01	0.00	0.01				

CHANNEL PROTECTION - 1 YEAR ENERGY BALANCE **OUTFALL 1 DEVELOPED CONDITIONS** d_{dev} RV_{pre} RV_{dev} COMMENTS

STORM P (inches) I.F. PRE-DEVELOPED CONDITIONS RCN_{off} DA_{off} RCN_w DA_t DA_t RCN_{on} DA_{on} RCN_{off} DA_{off} RCN_w RCN_{on} DA_{on} 94.65 2.61 0 0.00 95 2.61 2.61 2.195 0.526 1.02 1.993 2.662 5.202 3.53 1.81 1.64

27.00

27.60

28.20

28.80

29.40

30.00

30.60

Where: 24-hour Rainfall From NOAA Atlas 14, Vol 2, Station ID 44-8737

I.F. = improvement factor (site > 1 acre = 0.80 else 0.90, or 1.0 for woods) RCN = runoff curve number

Drainage Area (acres) potential maximum retention after runoff begins = 1000/RCN_w -10 runoff depth = $(P-0.2S)^2/(P+0.8S)$

runoff volume = DA, x d

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00 0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

0.00

pre-development peak flow rate from HEC-1 model allowable developed peak flow rate (Qaflow ≤ I.F. (Qpre x RVpre) / RVdev)

provided peak flow rate from prop. SWM pond

weighted total

B Dewberry

Dewberry Engineers, Inc 1503 Edwards Ferry Road Leesburg, VA 20176-6680

Ehlert Bryan

8609 Westwood Center Drive Tysons, VA 22182 703.827.9552

1306 Concourse Dr

Mueller Associates, Inc.

Linthicum, Maryland 21090 410.646.4500

NTE ON

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KEY PLAN

SCALE

NO SCALE

ADDENDUM 4 ADDENDUM 2 11/4/20 1 ADDENDUM 1 9/21/20 PERMIT SET DESCRIPTION

APPROVED B

1-YR ENERGY BALANCE **CALCULATIONS**

PROJECT NO.

C-06.202

Page 1

Summary for Link O-1: OUTFALL-1

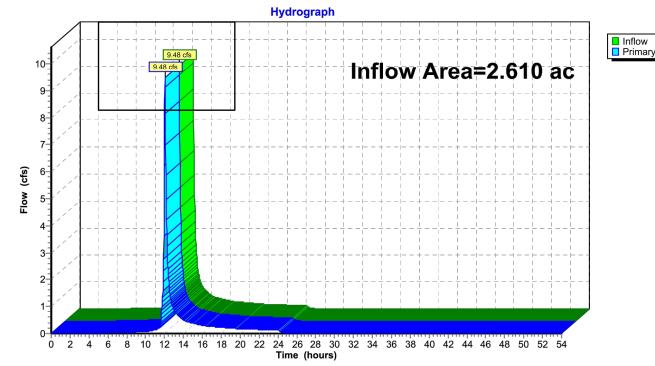
Inflow Area = 2.610 ac, 81.23% Impervious, Inflow Depth = 2.69" for 10 year event 9.48 cfs @ 12.04 hrs, Volume= Primary = 9.48 cfs @ 12.04 hrs, Volume= 0.586 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-54.00 hrs, dt= 0.03 hrs

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Prepared by DEWBERRY





OUTFALL-1-TRANSIT CENTER-REV1 Type II 24-hr 10 year Rainfall=4.61" Printed 11/16/2020 Prepared by DEWBERRY HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC

Hydrograph for Link O-1: OUTFALL-1

			- ·				
Time	Inflow	Elevation	Primary	Time	Inflow	Elevation	Primary
(hours)	(cfs)	(feet)	(cfs)	(hours)	(cfs)	(feet)	(cfs)
0.00	0.00	0.00	0.00	31.20	0.00	0.00	0.00
0.60	0.00	0.00	0.00	31.80	0.00	0.00	0.00
1.20	0.00	0.00	0.00	32.40	0.00	0.00	0.00
1.80	0.00	0.00	0.00	33.00	0.00	0.00	0.00
2.40	0.00	0.00	0.00	33.60	0.00	0.00	0.00
3.00	0.00	0.00	0.00	34.20	0.00	0.00	0.00
3.60	0.01	0.00	0.01	34.80	0.00	0.00	0.00
4.20	0.01	0.00	0.01	35.40	0.00	0.00	0.00
4.80	0.01	0.00	0.01	36.00	0.00	0.00	0.00
5.40	0.02	0.00	0.02	36.60	0.00	0.00	0.00
6.00	0.02	0.00	0.02	37.20	0.00	0.00	0.00
6.60	0.02	0.00	0.02	37.80	0.00	0.00	0.00
7.20	0.03	0.00	0.03	38.40	0.00	0.00	0.00
7.80	0.03	0.00	0.03	39.00	0.00	0.00	0.00
8.40	0.04	0.00	0.04	39.60	0.00	0.00	0.00
9.00	0.05	0.00	0.05	40.20	0.00	0.00	0.00
9.60	0.06	0.00	0.06	40.80	0.00	0.00	0.00
10.20	0.08	0.00	0.08	41.40	0.00	0.00	0.00
10.80	0.12	0.00	0.12	42.00	0.00	0.00	0.00
11.40	0.21	0.00	0.21	42.60	0.00	0.00	0.00
12.00	8.48	0.00	8.48	43.20	0.00	0.00	0.00
12.60	1.38	0.00	1.38	43.80	0.00	0.00	0.00
13.20	0.76	0.00	0.76	44.40	0.00	0.00	0.00
13.80	0.55	0.00	0.55	45.00	0.00	0.00	0.00
14.40	0.43	0.00	0.43	45.60	0.00	0.00	0.00
15.00	0.38	0.00	0.38	46.20	0.00	0.00	0.00
15.60	0.33	0.00	0.33	46.80	0.00	0.00	0.00
16.20	0.28	0.00	0.28	47.40	0.00	0.00	0.00
16.80	0.26	0.00	0.26	48.00	0.00	0.00	0.00
17.40	0.24	0.00	0.24	48.60	0.00	0.00	0.00
18.00	0.22	0.00	0.22	49.20	0.00	0.00	0.00
18.60	0.21	0.00	0.21	49.80	0.00	0.00	0.00
19.20	0.19	0.00	0.19	50.40	0.00	0.00	0.00
19.80	0.17	0.00	0.17	51.00	0.00	0.00	0.00
20.40	0.17	0.00	0.17	51.60	0.00	0.00	0.00
21.00	0.15	0.00	0.15	52.20	0.00	0.00	0.00
21.60	0.15	0.00	0.15	52.80	0.00	0.00	0.00
22.20	0.13	0.00	0.13	53.40	0.00	0.00	0.00
22.80	0.14	0.00	0.14	54.00	0.00	0.00	0.00
23.40	0.14	0.00	0.14	34.00	0.00	0.00	0.00
24.00	0.14	0.00	0.14				
24.60	0.13	0.00	0.13				
25.20	0.03	0.00	0.03				
25.20 25.80	0.01	0.00	0.01				
26.40			0.00				
26.40 27.00	0.00 0.00	0.00 0.00	0.00				
27.60 27.60	0.00		0.00				
		0.00					
28.20	0.00	0.00	0.00				
28.80	0.00	0.00	0.00				
29.40	0.00	0.00	0.00				
30.00	0.00	0.00	0.00				
30.60	0.00	0.00	0.00				

POST DEVELOPMENT PEAK FLOW RATE FOR PROPOSED IMPROVEMENTS YIELDS A DISCHARGE OF 9.48 CFS.

PRE DEVELOPMENT PEAK FLOW RATE FOR PRE-DEVELOPMENT YIELDS A DISCHARGE OF 9.53 CFS.

PER THESE CALCULATIONS, AND PER 9VAC-25-870-66.C.2.b, SINCE THE POST DEVELOPMENT PEAK FLOW IS LESS THAN THE PRE DEVELOPMENT PEAK FLOW, PROJECT COMPLIES WITH CHANNEL PROTECTION AND WATER QUANTITY REQUIREMENTS, SEE NARRATIVE ON SHEET C-06.205.



Dewberry Engineers, Inc 1503 Edwards Ferry Road Leesburg, VA 20176-6680 703.771.8004

Ehlert Bryan

8609 Westwood Center Drive Tysons, VA 22182 703.827.9552

Mueller Associates, Inc.

1306 Concourse Dr

Linthicum, Maryland 21090 410.646.4500

KEY PLAN

REVISIONS

NO SCALE

3	ADDENDUM 4	12/3/20
2	ADDENDUM 2	11/4/20
	4	

1 ADDENDUM 1 0 PERMIT SET

APPROVED BY

DECEMBER, 2020

PRE & POST 10-YR **EVENT SWM CALCULATIONS**

PROJECT NO.

C-06.203

10-YEAR STORM EVENT ROUTINGS Type II 24-hr 10 year Rainfall=4.61" **OUTFALL-1-TRANSIT CENTER-REV1** Printed 11/16/2020 Prepared by DEWBERRY HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC

Summary for Subcatchment PD: PRE-DEV

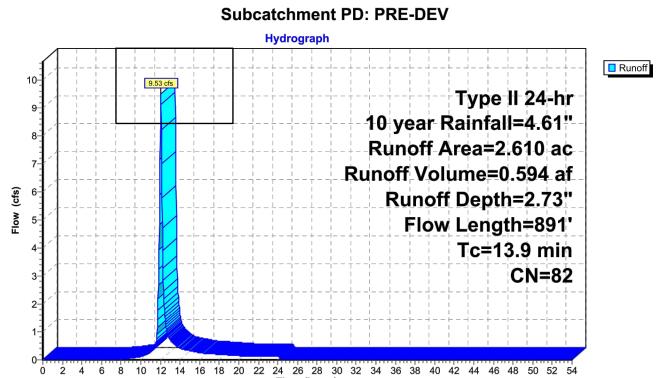
= 9.53 cfs @ 12.06 hrs, Volume= 0.594 af, Depth= 2.73" Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-54.00 hrs, dt= 0.03 hrs Type II 24-hr 10 year Rainfall=4.61"

Area (ac) CN Description 2.290 80 >75% Grass cover, Good, HSG D 0.320 98 Paved parking, HSG D 2.610 82 Weighted Average 2.290 87.74% Pervious Area

0.320 12.26% Impervious Area Tc Length Slope Velocity Capacity Description

(min) (feet) (ft/ft) (ft/sec) 6.3 50 0.0160 0.13 Sheet Flow, E-1 Grass: Short n= 0.150 P2= 3.05" 796 0.0122 1.78 Shallow Concentrated Flow, E-2 Unpaved Kv= 16.1 fps Shallow Concentrated Flow, E-3 45 0.3300 9.25 Unpaved Kv= 16.1 fps

13.9 891 Total



OUTFALL-1-TRANSIT CENTER-REV1 Prepared by DEWBERRY

Type II 24-hr 10 year Rainfall=4.61" Printed 11/16/2020

Page 1

HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC Hydrograph for Subcatchment PD: DRE-DEV

			Hydrogra	ph for S	ubcatcl	nment P	D: PRE-DEV
Time	Precip.	Excess	Runoff	Time	Precip.	Excess	Runoff
(hours)	(inches)	(inches)	(cfs)	(hours)	(inches)	(inches)	(cfs)
0.00	0.00	0.00	0.00	31.20	4.61	2.73	0.00
0.60	0.03	0.00	0.00	31.80	4.61	2.73	0.00
1.20	0.06	0.00	0.00	32.40	4.61	2.73	0.00
1.80	0.09	0.00	0.00	33.00	4.61	2.73	0.00
2.40	0.12	0.00	0.00	33.60	4.61	2.73	0.00
3.00	0.16	0.00	0.00	34.20	4.61	2.73	0.00
3.60	0.20	0.00	0.00	34.80	4.61	2.73	0.00
4.20	0.23	0.00	0.00	35.40	4.61	2.73	0.00
4.80	0.28	0.00	0.00	36.00	4.61	2.73	0.00
5.40	0.32	0.00	0.00	36.60	4.61	2.73	0.00
6.00	0.37	0.00	0.00	37.20	4.61	2.73	0.00
6.60	0.42	0.00	0.00	37.80	4.61	2.73	0.00
7.20	0.48	0.00	0.00	38.40	4.61	2.73	0.00
7.80	0.53	0.00	0.02	39.00	4.61	2.73	0.00
8.40	0.60	0.01	0.03	39.60	4.61	2.73	0.00
9.00	0.68	0.02	0.06	40.20	4.61	2.73	0.00
9.60	0.77	0.04	0.09	40.80	4.61	2.73	0.00
10.20	0.87	0.07	0.14	41.40	4.61	2.73	0.00
10.80	1.02	0.12	0.24	42.00	4.61	2.73	0.00
11.40	1.25	0.12	0.47	42.60	4.61	2.73	0.00
12.00	3.06	1.42	8.36	43.20	4.61	2.73	0.00
12.60	3.43	1.72	1.16	43.80	4.61	2.73	0.00
13.20	3.43	1.88	0.63	44.40	4.61	2.73	0.00
13.80	3.74	1.99	0.46	45.00	4.61	2.73	0.00
14.40	3.85	2.07	0.46	45.60	4.61	2.73	0.00
15.00	3.93	2.15	0.32	46.20	4.61	2.73	0.00
15.60	4.01	2.13	0.32	46.80	4.61	2.73	0.00
16.20	4.01	2.27	0.26	47.40	4.61	2.73	0.00
16.20	4.06	2.32	0.24	48.00	4.61	2.73	0.00
17.40		2.37		48.60		2.73	0.00
18.00	4.19	2.37	0.21 0.19		4.61 4.61		0.00
	4.25			49.20		2.73	
18.60	4.29	2.46	0.18	49.80	4.61	2.73	0.00
19.20	4.34	2.49	0.16	50.40	4.61	2.73	0.00
19.80	4.38	2.53	0.15	51.00	4.61	2.73	0.00
20.40	4.41	2.56	0.14	51.60	4.61	2.73	0.00
21.00	4.45	2.59	0.13	52.20	4.61	2.73	0.00
21.60	4.48	2.62	0.13	52.80	4.61	2.73	0.00
22.20	4.51	2.65	0.13	53.40	4.61	2.73	0.00
22.80	4.55	2.68	0.12	54.00	4.61	2.73	0.00
23.40	4.58	2.71	0.12				
24.00	4.61	2.73	0.12				
24.60	4.61	2.73	0.00				
25.20	4.61	2.73	0.00				
25.80	4.61	2.73	0.00				
26.40	4.61	2.73	0.00				
27.00	4.61	2.73	0.00				
27.60	4.61	2.73	0.00				
28.20	4.61	2.73	0.00				
28.80	4.61	2.73	0.00				
29.40	4.61	2.73	0.00				
30.00	4.61	2.73	0.00				
30.60	4.61	2.73	0.00				

POST DEVELOPMENT 10-YEAR STORM EVENT-BIORETENTION ROUTING

OUTFALL-1-TRANSIT CENTER-REV1 Type II 24-hr 10 year Rainfall=4.61" Printed 11/17/2020 Prepared by DEWBERRY HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC Page 1 **Summary for Pond Bio-1: BIO-1**

2.110 ac, 82.46% Impervious, Inflow Depth = 4.03" for 10 year event 13.07 cfs @ 11.97 hrs, Volume= 0.709 af 7.95 cfs @ 12.06 hrs, Volume= 0.422 af, Atten= 39%, Lag= 5.1 min 7.95 cfs @ 12.06 hrs, Volume= 0.422 af Primary =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-54.00 hrs, dt= 0.03 hrs Peak Elev= 290.75' @ 12.06 hrs Surf.Area= 7,862 sf Storage= 15,643 cf

Plug-Flow detention time= 212.5 min calculated for 0.422 af (60% of inflow) Center-of-Mass det. time= 105.8 min (873.2 - 767.5)

Invert Avail.Storage Storage Description

				3		
	#1	282.50'	17,697 cf	Custom Stage	Data (Prismatic)Listed below (Recalc)	
E	Elevation	Surf.Area		Inc.Store	Cum.Store	
_	(feet)	(sq-ft)	(%)	(cubic-feet)	(cubic-feet)	
	282.50	2,073	0.0	0	0	
	283.50	2,489	40.0	912	912	
	285.00	3,121	40.0	1,683	2,595	
	285.67	3,409		875	3,470	
	286.00	3,552		287	3,758	
	288.00	4,434		1,997	5,754	
	289.50	5,115		1,790	7,544	
	290.00	6,258		2,843	10,388	
	290.50	7,248		3,377	13,764	
	291.00	8.482	100.0	3.933	17.697	

Invert Outlet Devices

282.50' **18.0" Round Culvert** L= 35.4' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 282.50' / 282.25' S= 0.0071 '/' Cc= 0.900

n= 0.013, Flow Area= 1.77 sf 290.32' **26.0" x 26.0" Horiz. Orifice/Grate** C= 0.600 Limited to weir flow at low heads

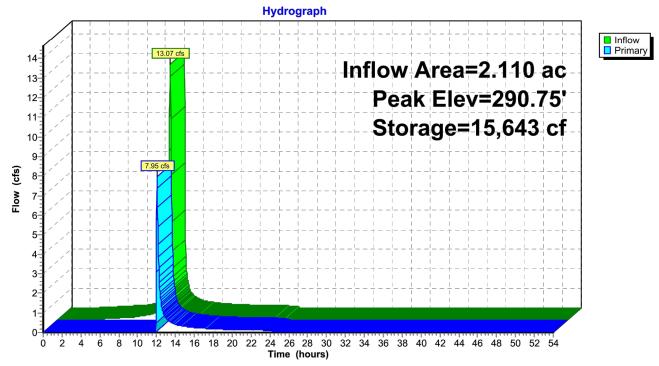
Primary OutFlow Max=7.92 cfs @ 12.06 hrs HW=290.75' TW=0.00' (Dynamic Tailwater)

1=Culvert (Passes 7.92 cfs of 28.38 cfs potential flow) **2=Orifice/Grate** (Weir Controls 7.92 cfs @ 2.14 fps)

Type II 24-hr 10 year Rainfall=4.61" **OUTFALL-1-TRANSIT CENTER-REV1**

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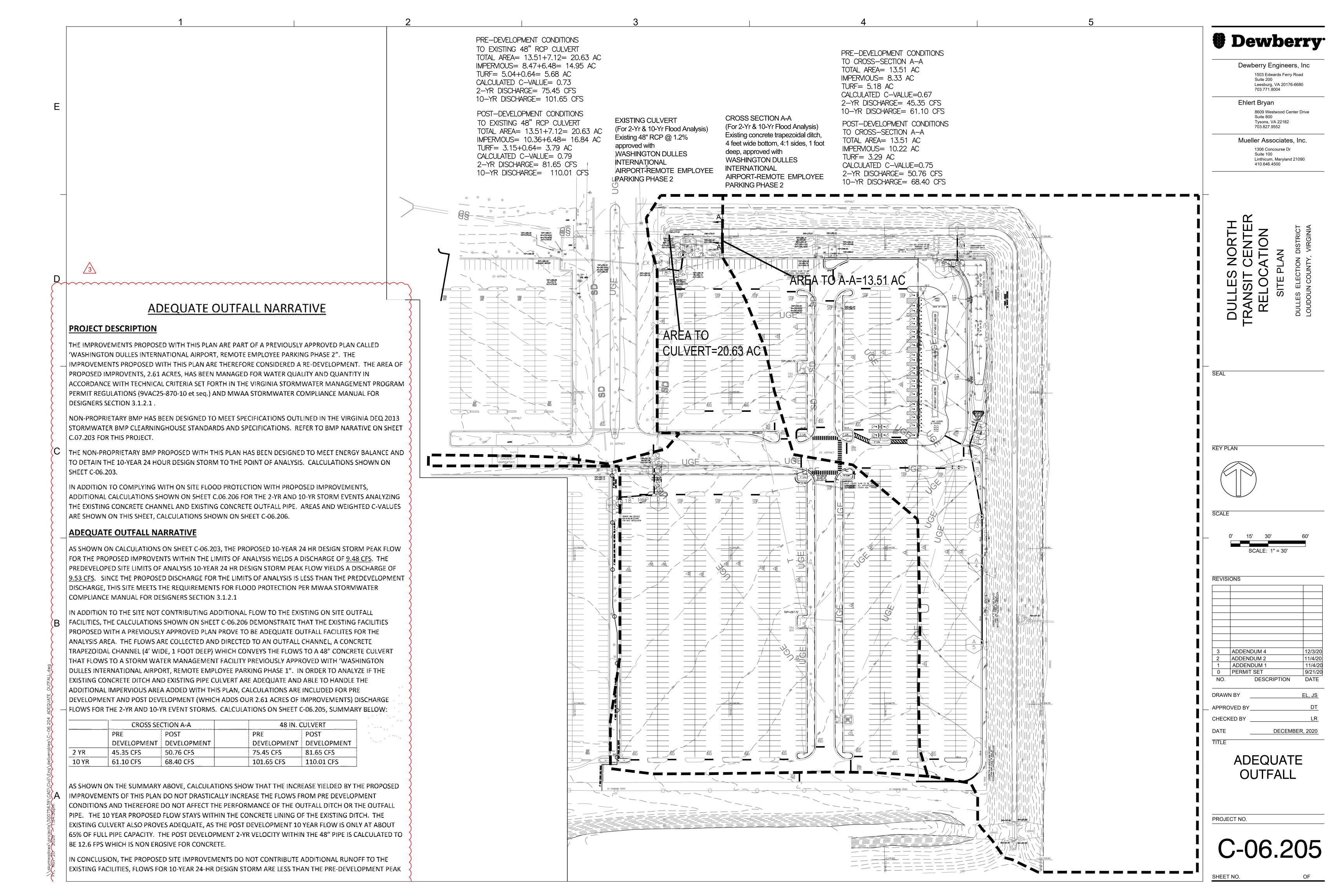
Pond Bio-1: BIO-1

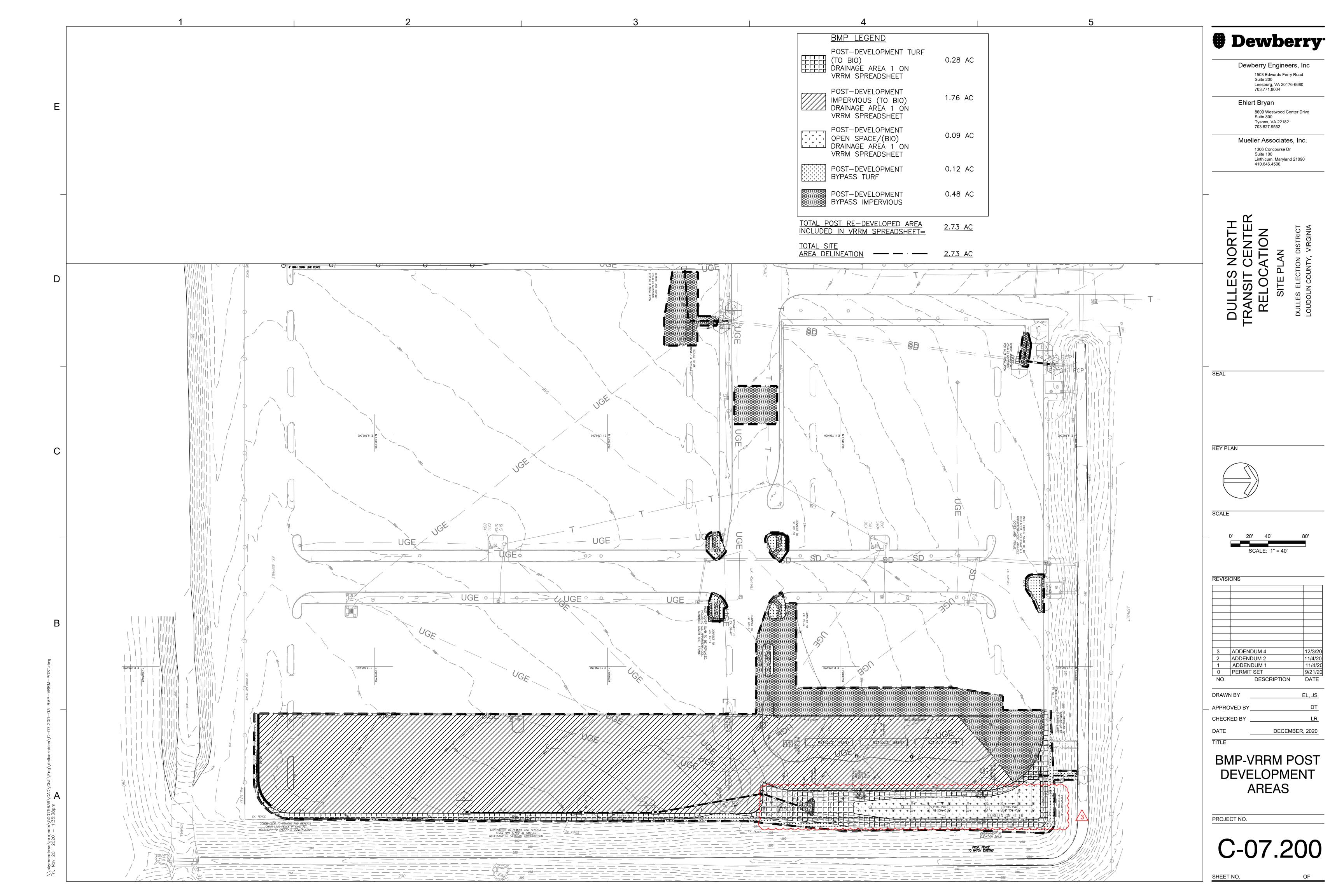


OUTFALL-1-TRANSIT CENTER-REV1 Type II 24-hr 10 year Rainfall=4.61" Prepared by DEWBERRY HydroCAD® 10.00-19 s/n 09482 © 2016 HydroCAD Software Solutions LLC

Hydrograph for Pond Bio-1: BIO-1

Time	Inflow	Storage	Elevation	Primary
(hours)	(cfs)	(cubic-feet)	(feet)	(cfs)
0.00	0.00	0	282.50	0.00
1.50	0.00	0	282.50	0.00
3.00	0.02	30	282.54	0.00
4.50	0.06	232	282.77	0.00
6.00	0.10	638	283.22	0.00
7.50	0.14	1,263	283.84	0.00
9.00	0.23	2,196	284.67	0.00
10.50	0.41	3,768	286.01	0.00
12.00	12.46	15,010	290.67	5.80
13.50	0.48	12,983	290.39	0.53
15.00	0.30	12,834	290.37	0.31
16.50	0.21	12,763	290.36	0.22
18.00	0.18	12,732	290.35	0.18
19.50	0.14	12,699	290.35	0.15
21.00	0.12	12,676	290.35	0.12
22.50	0.11	12,668	290.35	0.12
24.00	0.11	12,661	290.34	0.11
25.50	0.00	12,516	290.32	0.01
27.00	0.00	12,501	290.32	0.00
28.50	0.00	12,496	290.32	0.00
30.00	0.00	12,494	290.32	0.00
31.50	0.00	12,493	290.32	0.00
33.00	0.00	12,493	290.32	0.00
34.50	0.00	12,493	290.32	0.00
36.00	0.00	12,492	290.32	0.00
37.50	0.00	12,492	290.32	0.00
39.00	0.00	12,492	290.32	0.00
40.50	0.00	12,492	290.32	0.00
42.00	0.00	12,492	290.32	0.00
43.50	0.00	12,492	290.32	0.00
45.00	0.00	12,492	290.32	0.00
46.50	0.00	12,492	290.32	0.00
48.00	0.00	12,492	290.32	0.00
49.50	0.00	12,492	290.32	0.00
51.00	0.00	12,492	290.32	0.00
52.50	0.00	12,492	290.32	0.00
54.00	0.00	12,492	290.32	0.00





REFER TO SPECIFICATIONS FOR LANDSCAPE REQUIREMENTS IN ADDITION TO THE FOLLOWING:

1. AS A MINIMUM, ALL PLANT MATERIAL TO BE NURSERY-GROWN STOCK AND MEET OR EXCEED CURRENT A.N.S.I. STANDARDS BY THE AMERICAN ASSOCIATION OF NURSERYMEN, INC.

2. ALL TREES AND SHRUBS TO BE TOP DRESSED WITH DOUBLE SHREDDED HARDWOOD MULCH AT 2" TO 4" THICK. DO NOT PLACE MULCH WITHIN 6" OF TRUNK

3. BIORETENTION FILTER MEDIA USED TO BACKFILL PLANTING PITS SHALL BE TESTED BY A LABORATORY RECOMMENDED BY THE USDA AND THE RESULTS SHALL BE SUBMITTED TO THE OWNER FOR APPROVAL AS A BASE LINE. THE MATERIAL SHALL BE AMENDED AS NEEDED TO PROVIDE THE FOLLOWING CHARACTERISTICS AS A MINIMUM: pH RANGE BETWEEN 5.5 AND 7, 5% ORGANIC MATTER AND 100% OF THE MATERIAL SHALL PASS A 2" SIEVE. ALL FILTER MEDIA USED TO BACKFILL PLANTING PITS SHALL BE AMENDED WITH 12-12-12 SLOW RELEASE FERTILIZER AT RATES RECOMMENDED BY THE MANUFACTURER. DATA SHEETS FOR ALL MATERIALS USED AS AMENDMENTS SHALL BE SUBMITTED TO THE OWNER FOR APPROVAL

4. ALL ROCK AND DEBRIS LARGER THAN ONE INCH (1") SHALL BE REMOVED FROM THE SIDES, BOTTOM, AND BACKFILL SOIL OF THE PLANTING PITS OF ALL PLANT MATERIALS. DISPOSE OF ALL DEBRIS OFF SITE.

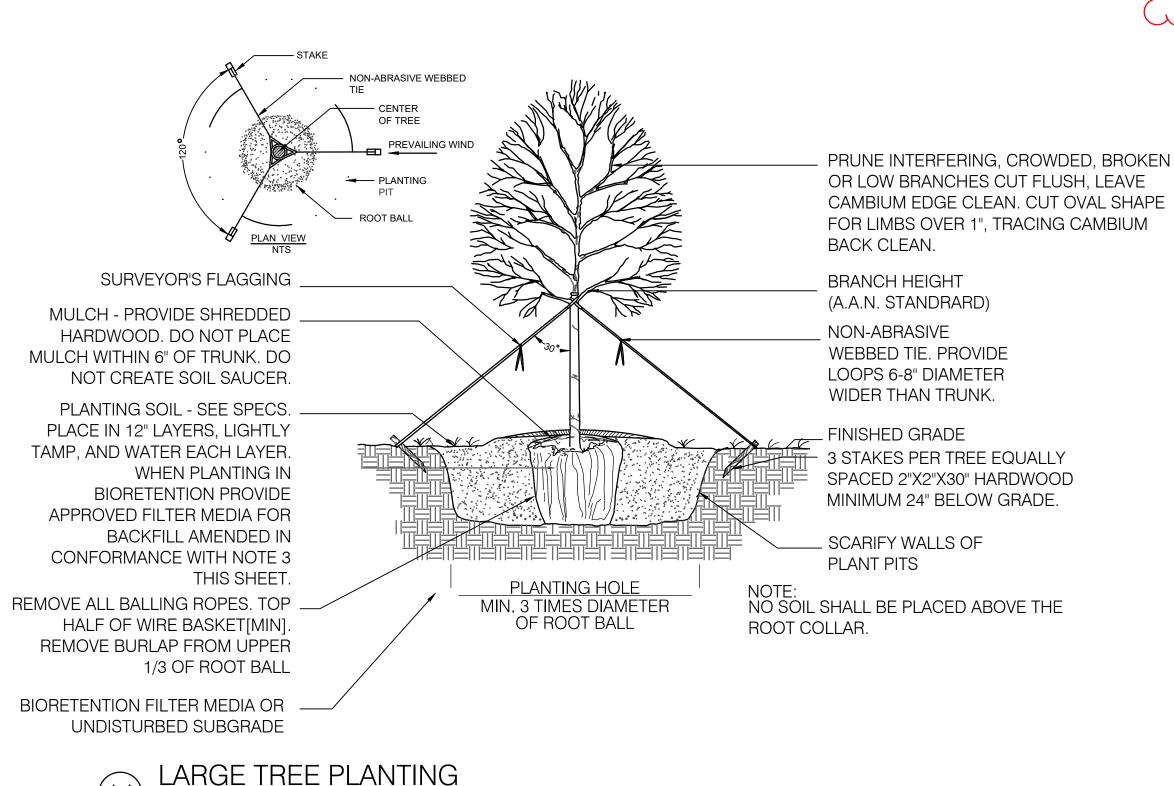
5. PLANT QUALITY: ALL SHRUBS SHALL BE DENSE, HEAVY TO THE GROUND AND WELL GROWN, SHOWING EVIDENCE OF HAVING BEEN SHEARED REGULARLY AND SHALL BE SOUND, FREE OF PLANT DISEASE OR INSECT EGGS AND SHALL HAVE A HEALTHY NORMAL ROOT SYSTEM. PLANTS SHALL BE FRESHLY DUG (WITHIN ONE GROWING SEASON) AND NOT HEELED-IN STOCK, NOR STOCK FROM COLD STORAGE. ALL PLANTS SHALL NOT BE IMMEDIATELY PRUNED PRIOR TO DELIVERY. THE SHAPE OF THE PLANT SHALL IN GENERAL CONFORM TO IS NATURAL GROWTH PROPORTIONS UNLESS OTHERWISE SPECIFIED. TREES SHALL CONFORM TO BRANCHING, CALIPER AND HEIGHT SPECIFICATIONS OF THE A.A.N. AND SHALL HAVE A WELL SHAPED, HEAVY BRANCH STRUCTURE FOR THE SPECIES. EVERGREEN TREES ARE TO HAVE AN INTERNODE NO GREATER THAN 18" AND SHALL BE UNIFORMLY WELL SHAPED.

6. THE CONTRACTOR WILL VERIFY PLANT QUANTITIES PRIOR TO BIDDING AND ANY DISCREPANCIES SHALL BE BROUGHT TO TH ATTENTION OF THE OWNER. THIS CONTRACT WILL BE BASED ON THE BIDDER HAVING VERIFIED, PRIOR TO BIDDING, THE AVAILABILITY OF THE REQUIRED PLANT MATERIALS AS SPECIFIED ON THE PLANT LIST

7. AS A MINIMUM. THE CONTRACTOR SHALL BE RESPONSIBLE DURING THE CONTRACT AND UP TO THE TIME OF ACCEPTANCE FOR KEEPING THE PLANTING AND WORK INCIDENTAL THERETO IN GOOD CONDITION. BY REPLANTING, PLANT REPLACEMENT WATERING, WEEDING, CULTIVATING, PRUNING AND SPRAYING, RESTAKING AND CLEANING UP AND PERFORMING ALL OTHER NECESSARY OPERATIONS OF CARE OF PROMOTION OF GOOD PLANT GROWTH SO THAT ALL WORK IS IN SATISFACTORY CONDITIONS AT THE TIME OF ACCEPTANCE, AT NO ADDITIONAL COST TO THE OWNER.

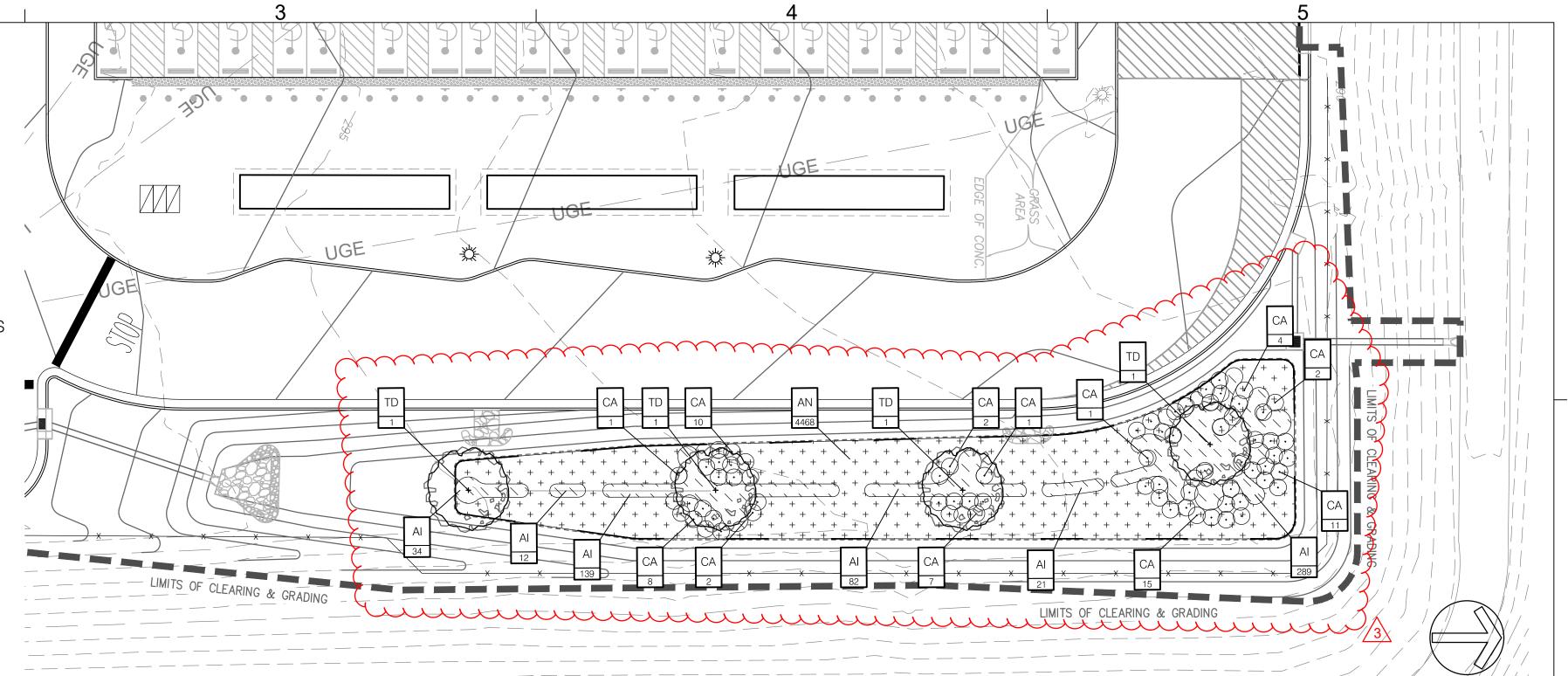
8. AS A MINIMUM, THE CONTRACTOR SHALL APPLY AND BLEND GRANULAR FERTILIZER TO THE SOIL MIX WITH A 10-6-4 ANALYSIS AT THE FOLLOWING RATES: TREE PITS, 1 LBS. PER CALIPER INCH; SHRUB BEDS, 3-5 LBS. PER 100 SQ. FT.; GROUND COVER, 2-3 LBS. PER 100 SQ. FT.

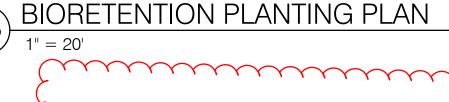
- 10. TREE PRESERVATION GUIDELINES AND TECHNIQUES TO CONFORM TO THE "VIRGINIA EROSION AND SEDIMENT CONTROL HANDBOOK," THIRD EDITION, 1992
- 11. ALL PLANTINGS IN OR ADJACENT TO WETLANDS ARE TO BE HAND DUG. THE LANDSCAPE CONTRACTOR SHALL NOT DISTURB ANY WETLAND AREAS. MACHINERY IS STRICTLY PROHIBITED IN OR IMMEDIATELY ADJACENT TO WETLAND AREAS. THE LANDSCAPE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING ALL WETLAND AREAS CLEARLY IDENTIFIED TO THE EXTENT
- 12. ALL AREAS TO BE PLANTED OUTSIDE OF SITE CLEARING AND GRADING LIMITS AND WITHIN EXISTING CRITICAL ROOT ZONES TO REMAIN SHALL BE CLEARED AND GRUBBED BY HAND. ALL CLEARING AND GRADING WITHIN THESE AREAS SHALL BE PERFORMED IN A MANNER THAT WILL NOT DAMAGE EXISTING TREES OR COMPACT THE SOILS. THE USE OF POWERED EQUIPMENT IS PROHIBITED. CLEARING AND GRUBBING SHALL INCLUDE BUT NOT BE LIMITED TO THE REMOVAL OF EXISTING FENCING, WOODY SPECIES, ALL INVASIVE SPECIES AND TRASH.

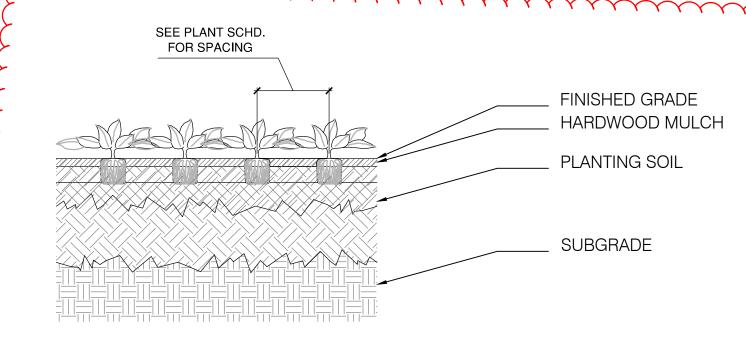


NOT TO SCALE

(2-1/2" CALIPER AND MORE)







PERENNIAL PLANTING NOT TO SCALE WATER THOROUGHLY TWICE minime the second secon WITHIN THE FIRST 48 HOURS.

MULCH - PROVIDE SHREDDED HARDWOOD, DO NOT PLACE MULCH WITHIN 6" OF STEMS/CANES. FINISHED GRADE

PLANTING SOIL - SEE SPECS. BACKFILL IN 6" LAYERS. LIGHTLY TAMP AND SETTLE EACH LAYER WITH WATER. WHEN PLANTING IN BIORETENTION PROVIDE APPROVED FILTER MEDIA FOR BACKFILL AMENDED IN CONFORMANCE WITH NOTE 3

NO SOIL SHALL BE PLACED

ABOVE THE ROOT COLLAR.

SHRUB PLANTING NOT TO SCALE

WATER THOROUGHLY TWICE WITHIN THE FIRST 48 HOURS.

THIS SHEET.

PLANT SCHEDULE

KE	EY	BOTANICAL NAME	COMMON NAME	PLNT QTY	SIZE	DETAIL	ROOTING	SPACING	REMARKS
χ _γ Τ	D	Taxodium distichum	BALD CYPRESS	4	3" C	A1	B+B	-	FULL MATCHING HEADS
	CA	Clethra alnifolia	SWEET PEPPERBUSH	64	3 GAL	C4	CONT	-	CANE COUNT SHALL MEET ANSI MINIMUMS WITHIN 2" OF ROOT COLLAR.
))				$\gamma \gamma \gamma \gamma$	Y Y Y	* * *)			
Σ \land	41	Asclepias incarnata 'Ice Ballet'	SWAMP MILKWEED	577	QUART	C3 .	CONT	18" O.C.	SPACING TO BE ON A TRIANGULAR GRID
	N	Aster novae-angliae 'Purple Dome'	NEW ENGLAND ASTER	4468	LP50	C3 .	FLAT	12" O.C.	SPACING TO BE ON A TRIANGULAR GRID
	N				LL.		\ \		
HEKBACE							3		
Ē									

Dewberry

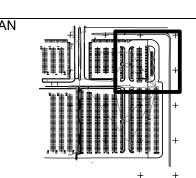
Ehlert Bryan

Dewberry Engineers, Inc

Leesburg, VA 20176-6680 703.771.8004

8609 Westwood Center Drive Tysons, VA 22182 703.827.9552

Mueller Associates, Inc Linthicum, Maryland 21090 410.646.4500



SCALE

REVISIONS

•		
3	ADDENDUM 4	12/3/20
2	ADDENDUM 2	11/4/20
1	ADDENDUM 1	11/4/20
0	PERMIT SET	9/21/20
NO.	DESCRIPTION	DATE
DRAW	N BY	EL, JS
APPRO	VED BY	DT

PLANTING PLAN, **NOTES & DETAILS**

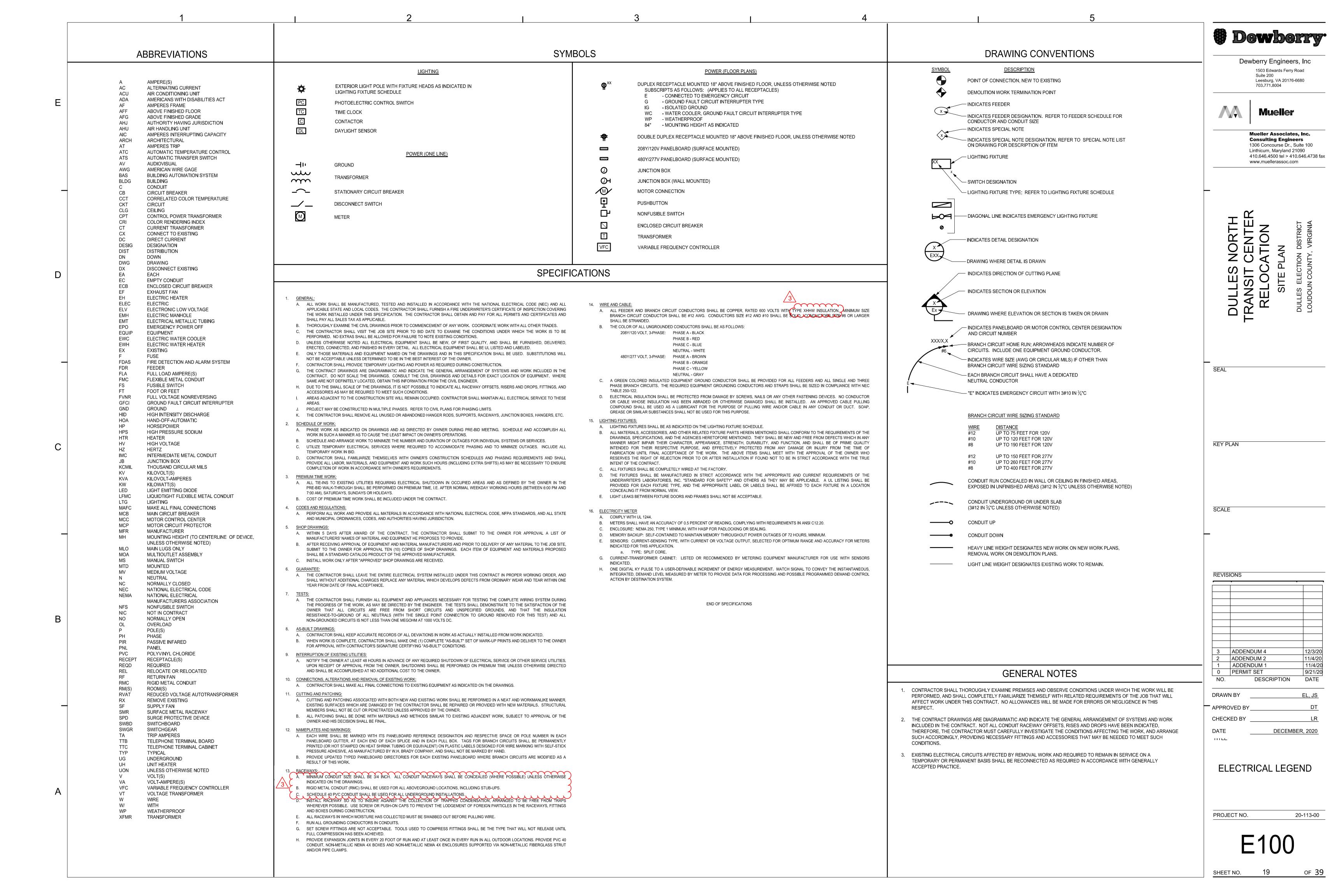
DECEMBER, 2020

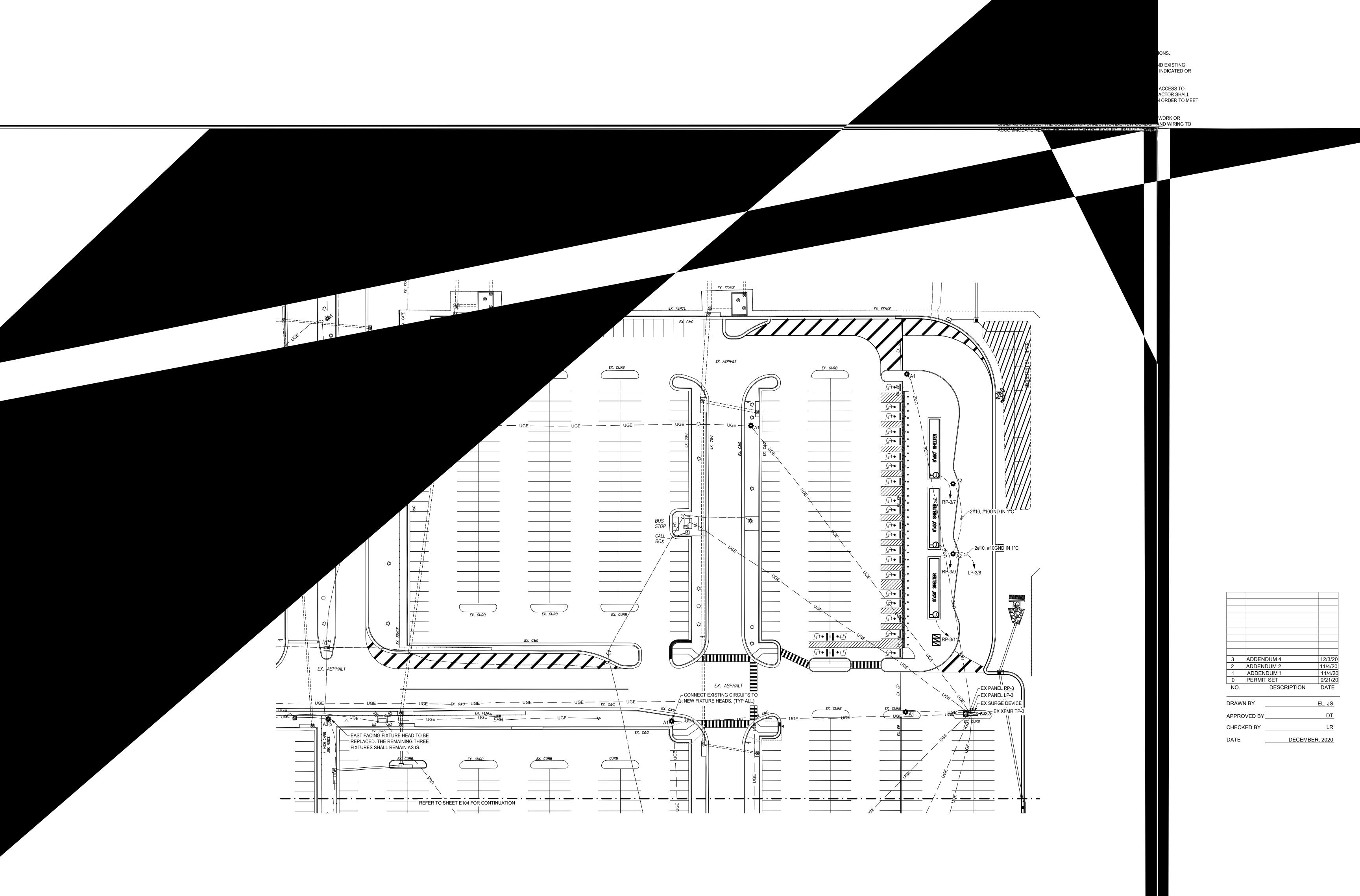
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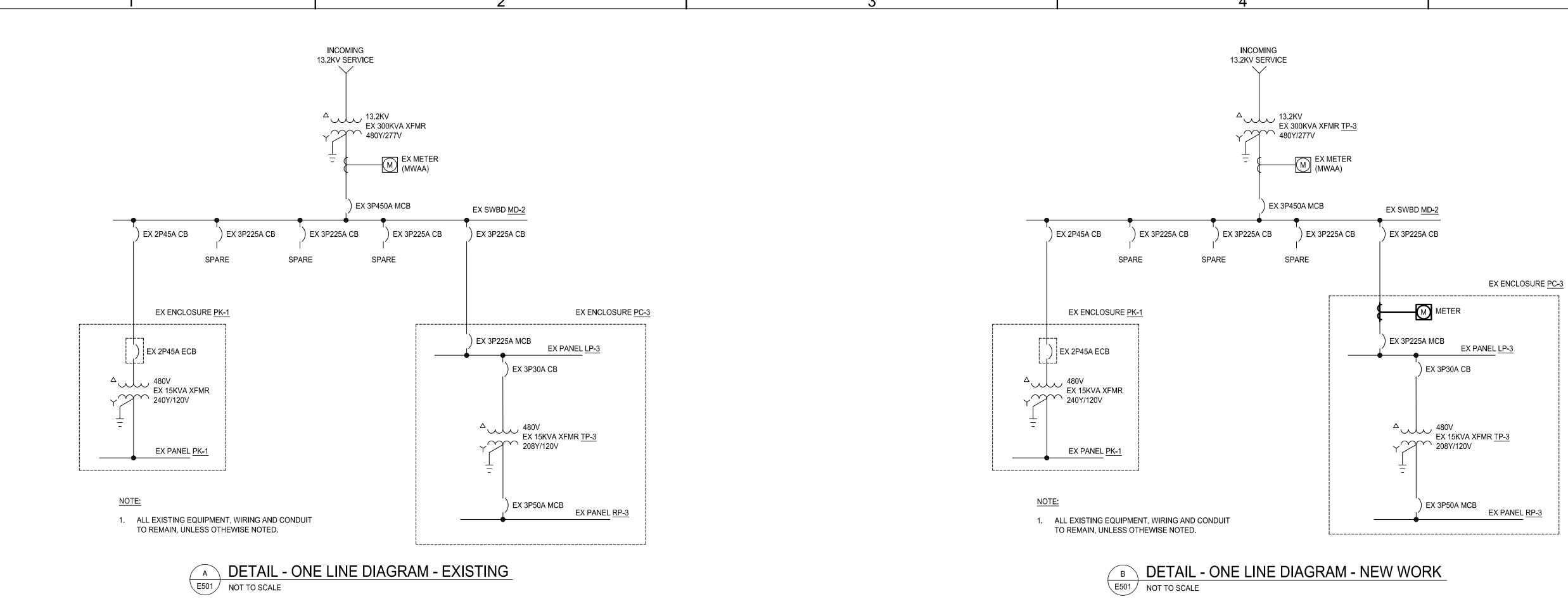
CHECKED BY

C-08.100

SHEET NO.







WARNING & IDENTIFICATION
TAPE LOCATED ALONG
ENTIRE LENGTH OF CONDUIT
& 12" ABOVE CONDUIT

BACKFILL
COMPACTED EARTH

CONCRETE
ENCASEMENT (3000 PSI)

WASHED GRAVEL FILL

C DETAIL - UNDERGROUND CONDUIT

NOT TO SCALE

EXISTING PANELBOARD: LP-3 MIN AIC: 14,000 NEMA 1 ENCLOSURE LOCATION: PC-3	BUS RATING: 250A VOLTS: 480Y/277V MOUNTING: SURFACE NOTES: MOUNTED INSIDE PC-3					}	MAIN: 3P225A MCB PHASES: 3 WIRES: 4 BRANCH CIRCUIT DEVICE: CB		
SERVES		В		CIRCUIT NUMBER		В	SERVES		
	Р	TA	INOIV	IDLIX	TA	Р			
EX MAST 49	1	30	1	2	30	1	EX MAST 45		
EX MAST 74	1	30	3	4	30	1	EX MAST 28		
EX MAST 47	1	30	5	6	30	1	EX MAST 25		
EX MAST 46	1	30	7	8	30	1	MAST 1A & 1B		
EX MAST 2	1	30	9	10	30	1	SPARE		
EX MAST 26	1	30	11	12	30	1	SPARE		
SPARE	1	30	13	14	30	1	SPARE		
SPARE	1	30	15	16	30	1	SPARE		
SPARE	1	30	17	18	30	1	SPARE		
SPARE	1	30	19	20	30	1	SPARE		
SPARE	1	30	21	22	30	1	SPARE		
SPARE	1	30	23	24	30	1	SPARE		
		3P2	00A CC	NTAC	TOR				
SPARE	1	30	25	26	30	1	SPARE		
SPARE	1	30	27	28	30	1	SPARE		
SPARE	1	30	29	30	30	1	SPARE		
			31	32					
SPARE	3	30	33	34	30	3	EX XFMR TP-3		
			35	36					
SPACE	1	-	37	38	-	1	SPACE		
SPACE	1	-	39	40	-	1	SPACE		
SPACE	1	-	41	42	-	1	SPACE		

EXISTING PANELBOARD: RP-3 MIN AIC: 10,000 NEMA 1 ENCLOSURE LOCATION: PC-3	VOLT MOUI	BUS RATING: 250A VOLTS: 208Y/120V MOUNTING: SURFACE NOTES: MOUNTED INSIDE PC-3					MAIN: 3P50A MCB PHASES: 3 WIRES: 4 BRANCH CIRCUIT DEVICE: CB	
SERVES	C	В		IRCUIT CB UMBER		В	SERVES	
	Р	TA	1 11011		TA	Р		
EX CCTV	1	20	1	2	20	1	EX CCTV	
EX BUS SHELTER LIGHTING	1	20	3	4	20	1	EX LIGHTING CONTROL	
EX EMERGENCY CALL LIGHT	1	20	5	6	20	1	EX EMERGENCY CALL LIGHT	
BUS SHELTER	1	20	7	8	20	1	EX POWER CENTER RECEPT	
BUS SHELTER	1	20	9	10	20	1	SPARE	
BUS SHELTER	1	20	11	12	20	1	SPARE	
SPARE	1	20	13	14	20	1	SPARE	
SPARE	1	20	15	16	20	1	SPARE	
SPARE	1	20	17	18	20	1	SPARE	
SPARE	1	20	19	20	20	1	SPARE	
SPARE	1	20	21	22	20	1	SPARE	
SPARE	1	20	23	24	20	1	SPARE	
SPARE	1	20	25	26	20	1	SPARE	
SPARE	1	20	27	28	20	1	SPARE	
SPARE	1	20	29	30	20	1	SPARE	

LIGHTING FIXTURES						
FIXTURE TYPE	MOUNTING	MANUFACTURER(S)	CATALOG OR MODEL NUMBER	LAMPS	VOLTS	REMARKS
A1	POLE MOUNTED	GARDCO	ECF-L-96L-800-NW-G2-5W	LED (INCLUDED)	277	FOUR POLE MOUNTED LIGHT FIXTURES MOUNTED ON EXISTING 50' TALL POLE. MEASURE MOUNTING PROVISIONS ON EXISTING POLE AND PROVIDE ADAPTERS AS REQUIRED TO MOUNT NEW FIXTURES.
A2	POLE MOUNTED	GARDCO	ECF-L-96L-800-NW-G2-5W	LED (INCLUDED)	277	TWO POLE MOUNTED LIGHT FIXTURES BACK TO BACK MOUNTED ON 30' TALL POLE.
А3	POLE MOUNTED	GARDCO	ECF-L-96L-800-NW-G2-5W	LED (INCLUDED)	277	ONE EAST FACING POLE MOUNTED LIGHT FIXTURE MOUNTED ON EXISTING 50' TALL POLE. MEASURE MOUNTING PROVISIONS ON EXISTING POLE AND PROVIDE ADAPTERS AS REQUIRED TO MOUNT NEW FIXTURES.



Dewberry Engineers, Inc 1503 Edwards Ferry Road Suite 200 Leesburg, VA 20176-6680 703.771.8004

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Mueller Associates

Mueller Associates, Inc.
Consulting Engineers
1306 Concourse Dr., Suite 100
Linthicum, Maryland 21090
410.646.4500 tel > 410.646.4738 fax
www.muellerassoc.com

DULLES NORTH
TRANSIT CENTER
RELOCATION
SITE PLAN

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SEAL

KEY PLAN

SCALE

REVISIONS

3 ADDENDUM 4 12/3/20
2 ADDENDUM 2 11/4/20
1 ADDENDUM 1 11/4/20
0 PERMIT SET 9/21/20
NO. DESCRIPTION DATE

DRAWN BY

APPROVED BY

CHECKED BY

DATE DECEMBER, 2020

ELECTRICAL DETAILS & SCHEDULES

PROJECT NO.

E501

20-113-00

SHEET NO. 19